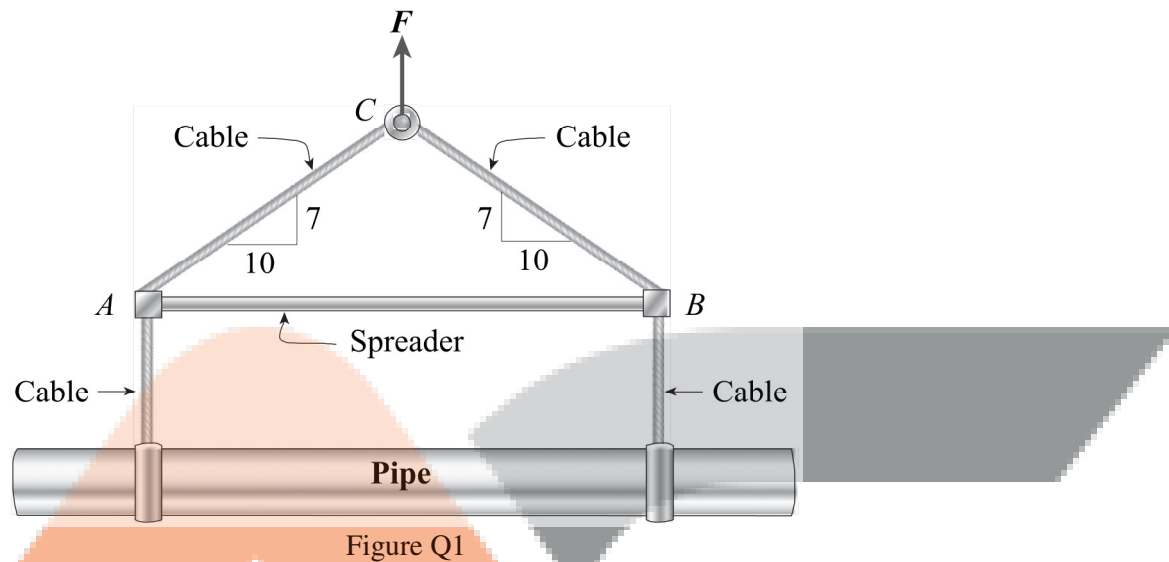


Problem 1

The hoisting arrangement for lifting a large pipe is shown in Figure Q1. The spreader is a steel tubular section with outer diameter 70 mm and inner diameter 57 mm. Its length is 2.6 m and its modulus of elasticity is 200 GPa. Evaluate the maximum weight of the pipe that can be lifted, considering a safety factor of 2.25 with respect to Euler buckling. Assume pinned conditions at the ends of the spreader. Hint: in the current arrangement, the spreader is under compression.

Hint: in the current arrangement, the spreader is under compression.



Solution

$$E = 200 \text{ GPa} \quad d_2 = 70 \text{ mm}$$

$$d_1 = 57 \text{ mm} \quad L = 2.6 \text{ m}$$

$$n = 2.25 \quad \alpha = \arctan\left(\frac{7}{10}\right)$$

$$I = \frac{\pi}{64} (d_2^4 - d_1^4) \quad I = 660.4 \times 10^3 \text{ mm}^4$$

$$P_{cr} = \frac{\pi^2 EI}{L^2} \quad P_{cr} = 199 \text{ kN}$$

ALLOWABLE LOAD

$$P_{allow} = \frac{P_{cr}}{n} \quad P_{allow} = 88.6 \text{ kN}$$

EQUILIBRIUM OF JOINT A

$$\Sigma F_{horiz} = 0 \quad -P + T \cos(\alpha) = 0$$

$$\Sigma F_{vert} = 0 \quad T \sin(\alpha) - \frac{W}{2} = 0$$

SOLVE THE EQUATION

$$W = 2P \tan(\alpha)$$

MAXIMUM WEIGHT OF PIPE

$$W_{max} = 2P_{allow} \tan(\alpha) \quad W_{max} = 124 \text{ kN}$$

Problem 2

Figure Q2 shows truss ABC which supports a vertical load W at joint B. Members AB and BC are made of circular hollow steel pipes with outside diameter 100 mm and wall thickness of 6 mm. Joint B is restrained against displacement perpendicular to the plane of the truss. Determine the critical buckling load, W_{cr} . Take Young's Modulus, $E = 200$ GPa.

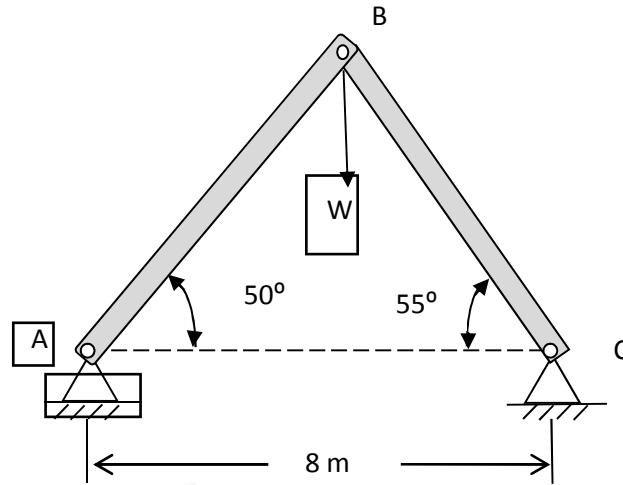
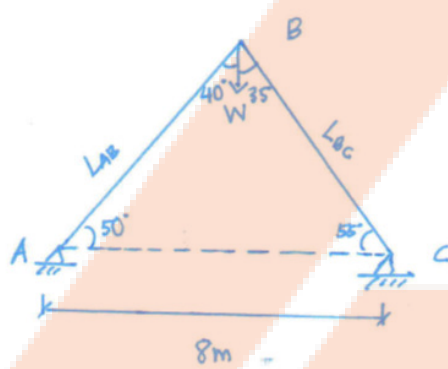


Figure Q2

Solution

Provided by Dr. Wong Jing Ying



$$d_2 = 100 \text{ mm}, \quad t = 6 \text{ mm}$$

$$d_1 = d_2 - 2t = 100 - 2(6) = 88 \text{ mm}$$

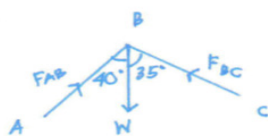
$$\frac{L_{AB}}{\sin 55} = \frac{8}{\sin 75}, \quad \frac{L_{BC}}{\sin 50} = \frac{8}{\sin 75}$$

$$L_{AB} = 6.784 \text{ m}, \quad L_{BC} = 6.345 \text{ m}$$

Buckling occurs when either member reaches its critical load.

$$P_{cr}^{AB} = \frac{\pi^2 EI}{L_{AB}^2} = \frac{\pi^2 (200)(1.96 \times 10^6)}{6.784^2} = 84.06 \text{ kN}$$

$$P_{cr}^{BC} = \frac{\pi^2 EI}{L_{BC}^2} = \frac{\pi^2 (200)(1.96 \times 10^6)}{6.345^2} = 96.1 \text{ kN}$$



$$\sum F_x = 0, \quad F_{AB} \sin 40 - F_{BC} \sin 35 = 0 \quad \text{--- (1)}$$

$$\sum F_y = 0, \quad F_{AB} \cos 40 + F_{BC} \cos 35 - W = 0 \quad \text{--- (2)}$$

$$\text{From ①, } F_{AB} = \frac{F_{BC} \sin 35}{\sin 40}, \quad F_{BC} = \frac{F_{AB} \sin 40}{\sin 35}$$

$$\Rightarrow \left(\frac{F_{BC} \sin 35}{\sin 40} \right) \cos 40 + F_{BC} \cos 35 - W = 0$$

$$W = 1.503 F_{BC}$$

$$\Rightarrow F_{AB} \cos 40 + \left(\frac{F_{AB} \sin 40}{\sin 35} \right) \cos 35 - W = 0$$

$$W = 1.684 F_{AB}$$

$$\therefore W_{CR}^{AB} = 1.684 P_{CR}^{AB} = 1.684 (84.06) = 141.56 \text{ kN}$$

$$W_{CR}^{BC} = 1.503 P_{CR}^{BC} = 1.503 (96.1) = 144.44 \text{ kN}$$